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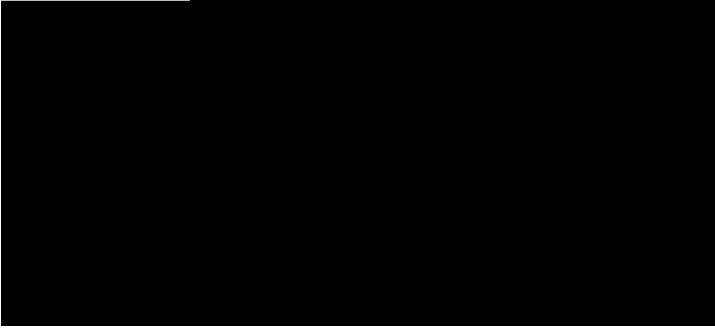
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Current Progress for HSM


- Member of National Cooperative Highway Research Program Project Panel 17 -68., “Intersection Crash Prediction Methods for the Highway Safety Manual.” Washington D.C., FY 2014-17



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5




Current Progress for HSM

- Member of National Cooperative Highway Research Program Project Panel 17 -74., “Developing Crash Modification Factors for Corridor Access Management “ Washington D.C., FY 2015-18

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6



Purpose and Need

- Why is a Calibration Factor needed?
 - f* Used to determine average predicted crash rates along sections of roadways/intersections
 - f* Method derived from Highway Safety Manual (HSM) which is based on lower 48 calibration data sets
 - f* Used to determine if a region calibration factor(s) are significantly different than lower 48 calibration factor values and if necessary as a design criteria


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Site Types Calibrated

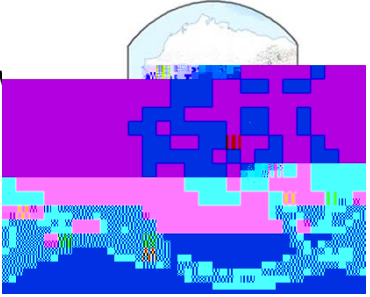
- Three- and Four-leg Stop-Controlled Intersections
- Four-leg Signalized Intersections
- Rural Two-lane Two-Way Highway Segments

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Regional Breakdown

- f*Central
 - Includes Anchorage Bo
- f*Northern
 - Includes Fairbanks
- f*Southcoast
 - Includes Ketchikan, Sitka, Juneau




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Regional Differences

- Potential Regional Differences:
 - f* Weather/Climate
 - f* AADT
 - f* Driver Behavior
 - f* Congestion
 - f* Wildlife
 - f* Daylight Hours
 - f* Crash Rates

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Methodology

- Acquire a list of sites to be calibrated
- Randomly sample sites until the appropriate sample size is reached (at least 100 crashes per intersection per year and at least 30 sites)
- Collect geometric information needed for Safety Performance Functions (SPFs)
 - f* In this study, this data was collected using Google Earth, Google Maps, site visits, MOA's map of AADT values, or was provided by the AKDOT&PF
- Apply HSM-given SPFs to find predicted crashes
- Apply the equation:

$$C = \frac{\sum \text{all sites observed crashes}}{\sum \text{SPFs}}$$


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Methodology

In some cases, the methodology used differed for the different facility types calibrated, or deviated from the HSM. These deviations are discussed next.

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Data Characteristics of Available Data

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Stop Control Intersections

All Regions

Definitions

U 3ST/4ST : Urban and Suburban Arterial 3-leg/4-leg Stop-Controlled Intersections

R2 3ST/4ST: Rural Two-Lane 3-leg/4-leg Stop- Controlled Intersections

RM 3ST/4ST: Rural Multilane 3-leg/4-leg Stop- Controlled Intersection

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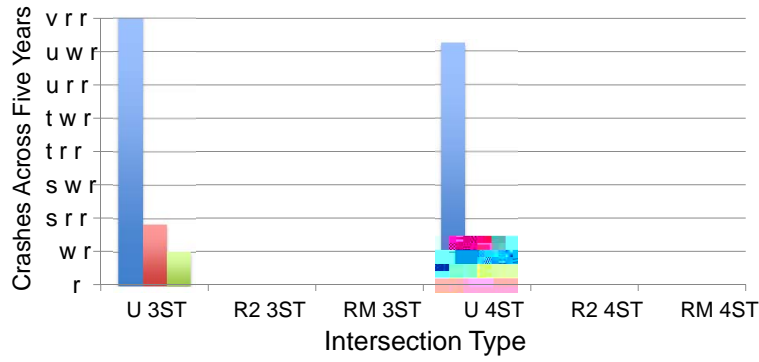
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Osama Abaza, 10/17/2016



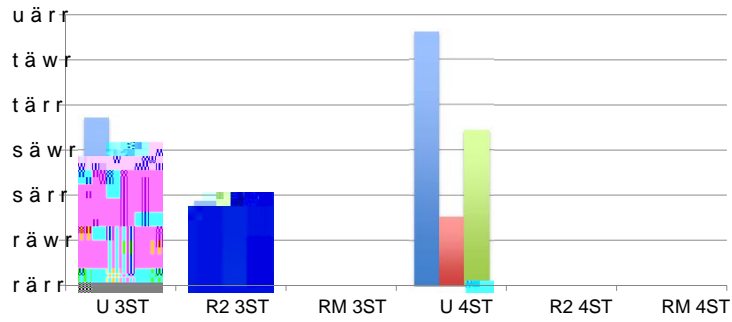
Crashes vs. Intersection Type For Stop-Controlled Intersections, 2008-2012



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Stop-Controlled Intersections Data Analysis

Major Street	Minor Street	Predicted Crashes	Observed Crashes
BADGER ROAD	NORDA RD	7	10
CHENA HO SPRINGS RD	NORDA RD	4	5
RICHARDSON HIGHWAY	JACK WARREN ROAD DELTA CT	5	3
RICHARDSON HIGHWAY	JOHNSON ROAD SALCHA	3	2
PARK HIGHWAY	LESTER ROAD HEALY	2	1
RICHARDSON HIGHWAY	DENAL HIGHWAY	1	0
MURPHY DOMER ROAD			



- f* Urban 3ST
 - CF = 1.72
- f* Urban 4ST
 - CF = 2.37
- f* Rural Two-Lane 3ST
 - CF = 0.82
- f* Rural Two-Lane 4ST
 - CF = 0.80
- f* Multilane intersections not calibrated due to lack of information



Stop-Controlled Intersections Validation


- Intersections not used for original calibration were then combined and another validation CF values found from these intersections
- Urban 3ST
 - f Validation CF value = 1.85
- Urban 4ST
 - f Validation CF value = 1.83
- Suggests higher CF values are indeed valid

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Major Street	Minor Street	Predicted Crashes	Observed Crashes
100TH AVENUE	KING STREET	2	7
15TH AVENUE	LAKETOWN PARKWAY	9	134
MINNESOTA DRIVE	26TH AVENUE	26	28
H STREET	4TH AVENUE	7	25
5TH AVENUE	CONCRETE STREET	28	61
5TH AVENUE	AIRPORT EIGHTH DRIVE	37	226
MULDOON ROAD	6TH AVENUE	21	88
88TH AVENUE	TOLOSON STREET	10	10
INT'L AIRPORT ROAD	ARCTIC BLVD	25	101
TUDOR ROAD	BAXTER ROAD/BEAVER PLACE	25	52
O'MALLEY ROAD	BIRCH ROAD	7	11
BONIFACE PARKWAY	NORTHERN NIGHTS BLVD	34	180
TUDOR ROAD	BONIFACE PARKWAY	26	109
MULDOON ROAD	BOUNDARY AVENUE	26	88
C STREET	POTTSDAM DRIVE	18	61


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Signalized Intersections Results

- Four-leg Signalized Intersection Calibration Factors:
 - f Central: 3.66
 - f Northern: 3.29
 - f Southcoast: 1.84

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Signalized Intersections Validation

- Central Region Validation Set of 46 Intersections Taken:

t-Test: Paired Two Sample for Means		
	N predicted	N observed
Mean	68.81	56.67
Variance	1260.67	2405.09
Observations	45	45
Pearson Correlation	0.67010693	
df	44	
t Stat	2.232	
P(T<=t) two-tail	0.031	< .05 so significant
t Critical two-tail	2.015	< t stat so significant


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Two-Lane Highway Segments Calibration Results

- Central Region
 - $fCF = 1.25$
- Northern Region
 - $fCF = 1.22$
- Combined
 - $fCF = 1.25$


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Conclusions

- Recommend developing Alaska-specific SPFs
- Some regional differences are estimated to affect Calibration Factor results more than others
 - f These include Average Annual Daily Traffic (AADT) and aggressive driving
- Minor regional differences that affect calibration results include:
 - f Wildlife, weather, daylight hours

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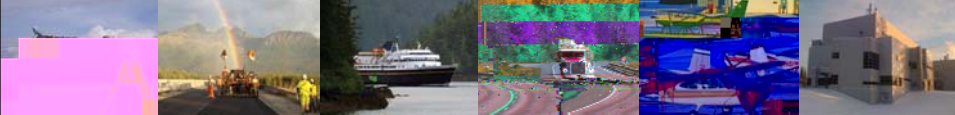
Recommendations

- For design use only, non enforcement
- Reference for analysis
- Update Calibration Factors every 4-6 years per the HSM
- New data set to use would be 2013-2017 crash data to compare different years

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Stop-Controlled Intersections Recommendations



Signalized Intersections Recommendations


Region	Recommended CF Value
Central	3.66
Northern	3.29
Southcoast	1.84

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- Recommended to use a CF value of 1.25 A().q.4in92






Acknowledgments

We thank the AKDOT&PF, along with the Federal Highway Administration for providing funding for this project. Thank you to those who performed research, helped with data collection, and helped in writing the report. Thanks to those at the AKDOT&PF who provided us with the necessary information to complete this project.

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39



Thank you for joining us today Questions?

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40